

- 1) GIVEN: The concrete column is reinforced with six $\text{Ø}1\text{-}1/8\text{in.}$ steel rods.

$$E_s = 29 \times 10^6 \text{ psi and } E_c = 4.2 \times 10^6 \text{ psi.}$$

REQ'D: Normal stresses in the steel and concrete if $P = 350 \text{ kips.}$ (B9.27)

$$\Delta L = \frac{P_c L}{E_c A_c} \Rightarrow P_c = \frac{E_c A_c \Delta L}{L}$$

$$\Delta L = \frac{P_s L}{E_s A_s} \Rightarrow P_s = \frac{E_s A_s \Delta L}{L}$$

$$P = P_c + P_s = (E_c A_c + E_s A_s) \epsilon$$

$$\epsilon = \frac{P}{E_c A_c + E_s A_s}$$

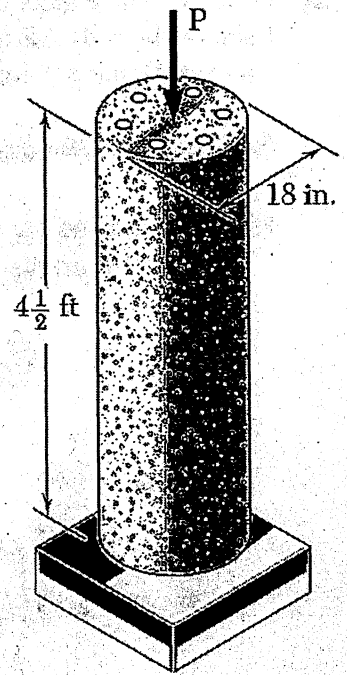
$$A_s = \frac{\pi}{4} (1.125 \text{ in})^2 \times 6 = 5.964 \text{ in}^2$$

$$A_c = \frac{\pi}{4} (18 \text{ in})^2 - 5.964 \text{ in}^2 = 248.5 \text{ in}^2$$

$$\begin{aligned} \epsilon &= \frac{-350 \times 10^3 \text{ lb (c)}}{4.2 \times 10^6 \text{ psi} (248.5 \text{ in}^2) + 29 \times 10^6 \text{ psi} (5.964 \text{ in}^2)} \\ &= \underline{\underline{-287.7 \times 10^{-6} \text{ in/in}}} \end{aligned}$$

$$\begin{aligned} \sigma_c &= E_c \epsilon = 4.2 \times 10^6 \text{ psi} (-287.7 \times 10^{-6} \text{ in/in}) \\ &= \underline{\underline{-1.21 \text{ ksi (c)}}} \end{aligned}$$

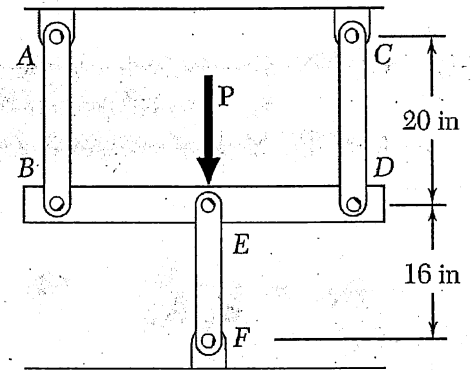
$$\begin{aligned} \sigma_s &= E_s \epsilon = 29 \times 10^6 \text{ psi} (-287.7 \times 10^{-6} \text{ in/in}) \\ &= \underline{\underline{-8.343 \text{ ksi (c)}}} \end{aligned}$$



- 2) GIVEN: Three steel rods ($E = 29 \times 10^6$ psi) support an 8.5-kip load P . Each of the rods AB and CD has a 0.32-in^2 cross-sectional area and rod EF has a 1-in^2 cross-sectional area. (B9.29)

Neglect any deformation of rod BED.

- REQ'D: (a) Find the change in length of rod EF
(b) Find the stress in each rod.



$$P_{AB} = P_{CD} \quad \text{By } \sum M_E = 0$$

$$\sum F_y = 0$$

$$P_{AB} + P_{CD} + P_{EF} - P = 0$$

$$P = 2P_{AB} + P_{EF}$$

SINCE BED IS RIGID

$$\delta_B = \delta_E = \delta_D$$

$$\frac{P_{AB} L_{AB}}{E A_{AB}} = \frac{P_{EF} L_{EF}}{E A_{EF}} \Rightarrow P_{AB} = P_{EF} \frac{L_{EF}}{L_{AB}} \frac{E A_{AB}}{E A_{EF}}$$

$$P_{AB} = P_{EF} \left(\frac{16}{20} \right) \left(\frac{0.32 \text{ in}^2}{1 \text{ in}^2} \right) = 0.256 P_{EF}$$

$$P = 2(0.256 P_{EF}) + P_{EF} \\ = 1.512 P_{EF}$$

$$P_{EF} = \frac{P}{1.512} = \frac{8.5 \text{ kips}}{1.512} = \underline{5.627 \text{ kips}}$$

$$P_{AB} = P_{CD} = 0.256 (5.627 \text{ kips}) = \underline{1.439 \text{ kips}}$$

$$a) \quad \Delta L_{EF} = \frac{P_{EF} L_{EF}}{A_{EF} E} = \frac{5.627 \text{ kips} (16 \text{ in})}{1 \text{ in}^2 (29 \times 10^6 \text{ psi})} = \underline{\underline{0.003106 \text{ in}}}$$

$$b) \quad \sigma_{AB} = \sigma_{CD} = \frac{P_{AB}}{A_{AB}} = \frac{1.439 \text{ kips}}{0.32 \text{ in}^2} = \underline{\underline{4.497 \text{ ksi}}}$$

$$\sigma_{EF} = \frac{P_{EF}}{A_{EF}} = \frac{5.627 \text{ kips}}{1 \text{ in}^2} = \underline{\underline{5.627 \text{ ksi}}}$$