

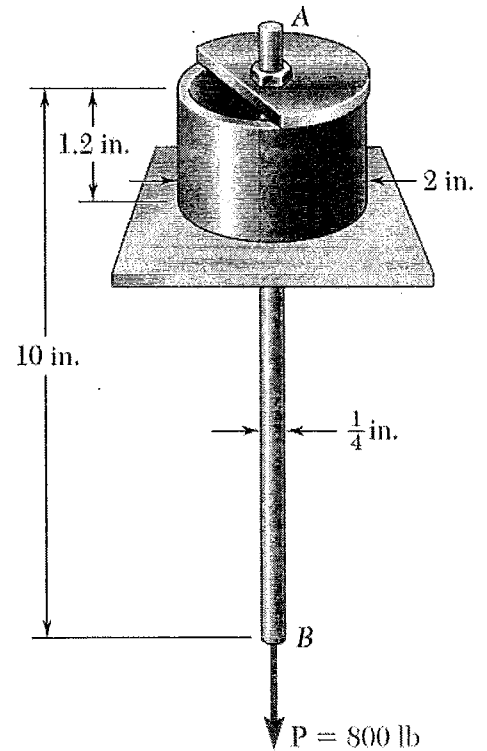
- 1) GIVEN: A 4.8ft long steel wire with $\varnothing 0.25$ in is subjected to a 750-lb tensile load. ($E_s = 29 \times 10^6$ psi)
 REQ'D: (a) Elongation of the wire.
 (b) Corresponding normal stress. (B9.1)

$$a) \Delta L = \frac{PL}{AE} = \frac{750(4.8\text{ft})(12 \text{ in/ft})}{\frac{\pi}{4}(0.25\text{in})^2 (29 \times 10^6 \text{ psi})}$$

$$= \underline{\underline{0.03035 \text{ in LONGER}}}$$

$$b) \sigma = \frac{P}{A} = \frac{750 \text{ lb}}{\frac{\pi}{4}(0.25\text{in})^2} = 15279 \text{ psi} \Rightarrow \underline{\underline{15.28 \text{ ksi (T)}}}$$

- 2) GIVEN: For electrical insulation a $\varnothing 0.25$ steel hanger is supported by a rigid circular plate (shown cutaway) resting on a $1/8$ in wall thickness polystyrene tube. ($E_{\text{poly}} = .45 \times 10^6$ psi)
 REQ'D: (a) Elongation of rod AB
 (b) Deflection of point B
 (c) Normal stress in rod AB. (B9.17)



a) FOR ROD AB

$$\Delta L_{AB} = \frac{P_{AB} L_{AB}}{A_{AB} E_{AB}} = \frac{800 \text{ lb} (10 \text{ in})}{\frac{\pi}{4}(0.25\text{in})^2 (29 \times 10^6 \text{ psi})}$$

$$= \underline{\underline{5.62 \times 10^{-3} \text{ in LONGER}}}$$

b) FOR POLY TUBE

$$A_{\text{TUBE}} = \frac{\pi}{4} (OD^2 - ID^2)$$

$$= \frac{\pi}{4} [(2 \text{ in})^2 - (2 - 2(0.125 \text{ in}))^2]$$

$$= 0.7363 \text{ in}^2$$

$$\Delta L_{\text{TUBE}} = \frac{P_{\text{T}} L_{\text{T}}}{A_{\text{T}} E_{\text{T}}} = \frac{-800 \text{ lb} (c) (1.2 \text{ in})}{0.7363 \text{ in}^2 (.45 \times 10^6 \text{ psi})}$$

$$= 2.90 \times 10^{-3} \text{ in SHORTER}$$

DEFLECTION AT B

$$\Delta L_B = \Delta L_{\text{TUBE}} + \Delta L_{AB}$$

$$= 5.62 \times 10^{-3} \text{ in} + 2.90 \times 10^{-3} \text{ in}$$

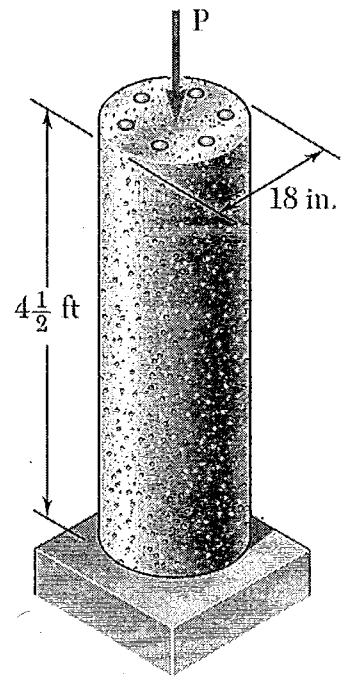
$$= \underline{\underline{8.52 \times 10^{-3} \text{ in LOWER}}}$$

STRESS IN AB

$$\sigma_{AB} = \frac{P_{AB}}{A_{AB}}$$

$$= \frac{800 \text{ lb}}{\frac{\pi}{4}(0.25\text{in})^2} = \underline{\underline{16.3 \text{ ksi (T)}}}$$

- 3) GIVEN: The concrete column is reinforced with six $\text{Ø}1\text{-}1/8\text{in.}$ steel rods.
 $E_s = 29 \times 10^6 \text{ psi}$ and $E_c = 4.2 \times 10^6 \text{ psi}$.
 REQ'D: Normal stresses in the steel and concrete if $P = 350\text{-kips}$. (B9.27)



$$\Delta L = \frac{P_c L}{E_c A_c} \quad P_c = \frac{E_c A_c \Delta L}{L}$$

$$\Delta L = \frac{P_s L}{E_s A_s} \quad P_s = \frac{E_s A_s \Delta L}{L}$$

$$P = P_c + P_s = (E_c A_c + E_s A_s) \frac{\Delta L}{L}$$

$$E = \frac{P}{E_c A_c + E_s A_s}$$

$$A_s = \frac{\pi}{4} (1.125 \text{ in})^2 \times 6 = 5.964 \text{ in}^2$$

$$A_c = \frac{\pi}{4} (18 \text{ in})^2 - \frac{A_s}{6} = 248.5 \text{ in}^2$$

$$E = \frac{-350 \times 10^3 \text{ lb (C)}}{(4.2 \times 10^6 \text{ psi})(248.5 \text{ in}^2) + (29 \times 10^6 \text{ psi})(5.964 \text{ in}^2)}$$

$$= -287.7 \times 10^{-6} \text{ in/in}$$

$$\sigma_c = E_c E = (4.2 \times 10^6 \text{ psi})(-287.7 \times 10^{-6}) = \underline{\underline{-1.20 \text{ ksi (C)}}$$

$$\sigma_s = E_s E = (29 \times 10^6 \text{ psi})(-287.7 \times 10^{-6}) = \underline{\underline{-8.34 \text{ ksi (C)}}$$

- 4) GIVEN: $\text{Ø}60\text{mm}$ bolts used to secure the top on a nuclear reactor vessel.
 REQ'D: Tension in bolts when $\Delta\text{Ø}$ is $13\mu\text{m}$. $E = 20\text{GPa}$ and $\nu = 0.29$

$$E_{\text{TRANS.}} = \frac{\Delta\text{Ø}}{\text{Ø}} = \frac{-13 \times 10^{-6} \text{ m}}{.060 \text{ m}} = -216.7 \times 10^{-6} \text{ m/m}$$

$$\nu = \frac{-E_{\text{TRANS.}}}{E_{\text{LONG.}}} \Rightarrow E_{\text{LONG.}} = -\frac{E_{\text{TRANS.}}}{\nu}$$

$$= \frac{216.7 \times 10^{-6}}{.29}$$

$$= 747.13 \times 10^{-6}$$

$$\sigma = E E$$

$$= (200 \times 10^9 \text{ Pa})(747.13 \times 10^{-6})$$

$$= 149.43 \times 10^6 \text{ Pa}$$

$$F = \sigma A$$

$$= 149.4 \times 10^6 \text{ Pa} \left(\frac{\pi}{4} (.06 \text{ m})^2 \right)$$

$$= 422 \times 10^3 \text{ N}$$

$$\Rightarrow \underline{\underline{422 \text{ kN (T)}}$$

